


## PROBLEM 1.1

Two solid cylindrical rods $A B$ and $B C$ are welded together at $B$ and loaded as shown. Knowing that $d_{1}=30 \mathrm{~mm}$ and $d_{2}=50 \mathrm{~mm}$, find the average normal stress at the midsection of $(a) \operatorname{rod} A B,(b) \operatorname{rod} B C$.

## SOLUTION

(a) $\operatorname{Rod} A B$ :

Force:

$$
P=60 \times 10^{3} \mathrm{~N} \text { tension }
$$

Area:

$$
A=\frac{\pi}{4} d_{1}^{2}=\frac{\pi}{4}\left(30 \times 10^{-3}\right)^{2}=706.86 \times 10^{-6} \mathrm{~m}^{2}
$$

Normal stress: $\quad \sigma_{A B}=\frac{P}{A}=\frac{60 \times 10^{3}}{706.86 \times 10^{-6}}=84.882 \times 10^{6} \mathrm{~Pa}$ $\sigma_{A B}=84.9 \mathrm{MPa}$
(b) $\operatorname{Rod} B C$ :

Force: $\quad P=60 \times 10^{3}-(2)\left(125 \times 10^{3}\right)=-190 \times 10^{3} \mathrm{~N}$
Area:

$$
A=\frac{\pi}{4} d_{2}^{2}=\frac{\pi}{4}\left(50 \times 10^{-3}\right)^{2}=1.96350 \times 10^{-3} \mathrm{~m}^{2}
$$

Normal stress: $\quad \sigma_{B C}=\frac{P}{A}=\frac{-190 \times 10^{3}}{1.96350 \times 10^{-3}}=-96.766 \times 10^{6} \mathrm{~Pa}$

$$
\sigma_{B C}=-96.8 \mathrm{MPa}
$$



## PROBLEM 1.2

Two solid cylindrical rods $A B$ and $B C$ are welded together at $B$ and loaded as shown. Knowing that the average normal stress must not exceed 150 MPa in either rod, determine the smallest allowable values of the diameters $d_{1}$ and $d_{2}$.

## SOLUTION

(a) $\operatorname{Rod} A B$ :

Force: $\quad P=60 \times 10^{3} \mathrm{~N}$
Stress: $\quad \sigma_{A B}=150 \times 10^{6} \mathrm{~Pa}$
Area:

$$
\begin{aligned}
A & =\frac{\pi}{4} d_{1}^{2} \\
\sigma_{A B} & =\frac{P}{A} \quad \therefore \quad A=\frac{P}{\sigma_{A B}} \\
\frac{\pi}{4} d_{1}^{2} & =\frac{P}{\sigma_{A B}} \\
d_{1}^{2} & =\frac{4 P}{\pi \sigma_{A B}}=\frac{(4)\left(60 \times 10^{3}\right)}{\pi\left(150 \times 10^{6}\right)}=509.30 \times 10^{-6} \mathrm{~m}^{2} \\
d_{1} & =22.568 \times 10^{-3} \mathrm{~m}
\end{aligned}
$$

$$
d_{1}=22.6 \mathrm{~mm}
$$

(b) $\operatorname{Rod} B C$ :

Force: $\quad P=60 \times 10^{3}-(2)\left(125 \times 10^{3}\right)=-190 \times 10^{3} \mathrm{~N}$
Stress: $\quad \sigma_{B C}=-150 \times 10^{6} \mathrm{~Pa}$
Area: $\quad A=\frac{\pi}{4} d_{2}^{2}$

$$
\sigma_{B C}=\frac{P}{A}=\frac{4 P}{\pi d_{2}^{2}}
$$

$$
d_{2}^{2}=\frac{4 P}{\pi \sigma_{B C}}=\frac{(4)\left(-190 \times 10^{3}\right)}{\pi\left(-150 \times 10^{6}\right)}=1.61277 \times 10^{-3} \mathrm{~m}^{2}
$$

$d_{2}=40.159 \times 10^{-3} \mathrm{~m}$
$d_{2}=40.2 \mathrm{~mm}$


## PROBLEM 1.3

Two solid cylindrical rods $A B$ and $B C$ are welded together at $B$ and loaded as shown. Knowing that $P=10 \mathrm{kips}$, find the average normal stress at the midsection of $(a) \operatorname{rod} A B,(b) \operatorname{rod} B C$.

## SOLUTION

(a) $\operatorname{Rod} A B$ :

$$
\begin{aligned}
P & =12+10=22 \mathrm{kips} \\
A & =\frac{\pi}{4} d_{1}^{2}=\frac{\pi}{4}(1.25)^{2}=1.22718 \mathrm{in}^{2} \\
\sigma_{A B} & =\frac{P}{A}=\frac{22}{1.22718}=17.927 \mathrm{ksi} \quad \sigma_{A B}=17.93 \mathrm{ksi}
\end{aligned}
$$

(b) $\operatorname{Rod} B C$ :

$$
\begin{aligned}
P & =10 \mathrm{kips} \\
A & =\frac{\pi}{4} d_{2}^{2}=\frac{\pi}{4}(0.75)^{2}=0.44179 \mathrm{in}^{2}
\end{aligned}
$$

$$
\sigma_{A B}=\frac{P}{A}=\frac{10}{0.44179}=22.635 \mathrm{ksi} \quad \sigma_{A B}=22.6 \mathrm{ksi}
$$



## PROBLEM 1.4

Two solid cylindrical rods $A B$ and $B C$ are welded together at $B$ and loaded as shown. Determine the magnitude of the force $\mathbf{P}$ for which the tensile stresses in rods $A B$ and $B C$ are equal.

## SOLUTION

(a) $\operatorname{Rod} A B$ :

$$
\begin{aligned}
P & =P+12 \mathrm{kips} \\
A & =\frac{\pi d^{2}}{4}=\frac{\pi}{4}(1.25 \mathrm{in} .)^{2} \\
A & =1.22718 \mathrm{in}^{2} \\
\sigma_{A B} & =\frac{P+12 \mathrm{kips}}{1.22718 \mathrm{in}^{2}}
\end{aligned}
$$

(b) $\operatorname{Rod} B C$ :

$$
\begin{array}{rl}
P= & P \\
A= & \frac{\pi}{4} d^{2}=\frac{\pi}{4}(0.75 \mathrm{in} .)^{2} \\
A= & 0.44179 \mathrm{in}^{2} \\
\sigma_{B C}= & \frac{P}{0.44179 \mathrm{in}^{2}} \\
\sigma_{A B}= & \sigma_{B C} \\
& \frac{P+12 \mathrm{kips}}{1.22718 \mathrm{in}^{2}}=\frac{P}{0.44179 \mathrm{in}^{2}} \\
5.3015=0.78539 P & P=6.75 \mathrm{kips}
\end{array}
$$



## SOLUTION

$$
\sigma=\frac{P}{A} \quad \therefore \quad A=\frac{P}{\sigma}
$$

Geometry: $\quad A=\frac{\pi}{4}\left(d_{1}^{2}-d_{2}^{2}\right)$

$$
\begin{aligned}
d_{2}^{2} & =d_{1}^{2}-\frac{4 A}{\pi}=d_{1}^{2}-\frac{4 P}{\pi \sigma} \\
d_{2}^{2} & =\left(25 \times 10^{-3}\right)^{2}-\frac{(4)(1200)}{\pi\left(3.80 \times 10^{6}\right)} \\
& =222.92 \times 10^{-6} \mathrm{~m}^{2}
\end{aligned}
$$

$$
d_{2}=14.93 \times 10^{-3} \mathrm{~m} \quad d_{2}=14.93 \mathrm{~m} \mathrm{~m}
$$

## PROBLEM 1.6



Two steel plates are to be held together by means of $16-\mathrm{mm}$ diameter high-strength steel bolts fitting snugly inside cylindrical brass spacers. Knowing that the average normal stress must not exceed 200 MPa in the bolts and 130 MPa in the spacers, determine the outer diameter of the spacers that yields the most economical and safe design.

## SOLUTION

At each bolt location the upper plate is pulled down by the tensile force $P_{b}$ of the bolt. At the same time, the spacer pushes that plate upward with a compressive force $P_{s}$ in order to maintain equilibrium.

$$
P_{b}=P_{s}
$$

For the bolt, $\quad \sigma_{b}=\frac{F_{b}}{A_{b}}=\frac{4 P_{b}}{\pi d_{b}^{2}} \quad$ or $\quad P_{b}=\frac{\pi}{4} \sigma_{b} d_{b}^{2}$
For the spacer, $\quad \sigma_{s}=\frac{P_{s}}{A_{s}}=\frac{4 P_{s}}{\pi\left(d_{s}^{2}-d_{b}^{2}\right)} \quad$ or $\quad P_{s}=\frac{\pi}{4} \sigma_{s}\left(d_{s}^{2}-d_{b}^{2}\right)$

Equating $P_{b}$ and $P_{s}$,

$$
\begin{aligned}
\frac{\pi}{4} \sigma_{b} d_{b}^{2} & =\frac{\pi}{4} \sigma_{s}\left(d_{s}^{2}-d_{b}^{2}\right) \\
d_{s} & =\sqrt{\left(1+\frac{\sigma_{b}}{\sigma_{s}}\right)} d_{b}=\sqrt{\left(1+\frac{200}{130}\right)}(16) \quad d_{s}=25.2 \mathrm{~mm}
\end{aligned}
$$



## SOLUTION

Use bar $A B C$ as a free body.

$\Sigma M_{C}=0:(0.040) F_{B D}-(0.025+0.040)\left(20 \times 10^{3}\right)=0$
$F_{B D}=32.5 \times 10^{3} \mathrm{~N} \quad$ Link $B D$ is in tension.
$\Sigma M_{B}=0:-(0.040) F_{C E}-(0.025)\left(20 \times 10^{3}\right)=0$

$$
F_{C E}=-12.5 \times 10^{3} \mathrm{~N} \quad \text { Link } C E \text { is in compression. }
$$

Net area of one link for tension $=(0.008)(0.036-0.016)=160 \times 10^{-6} \mathrm{~m}^{2}$
For two parallel links, $\quad A_{\text {net }}=320 \times 10^{-6} \mathrm{~m}^{2}$
(a) $\sigma_{B D}=\frac{F_{B D}}{A_{\text {net }}}=\frac{32.5 \times 10^{3}}{320 \times 10^{-6}}=101.563 \times 10^{6}$ $\sigma_{B D}=101.6 \mathrm{MPa}$

Area for one link in compression $=(0.008)(0.036)=288 \times 10^{-6} \mathrm{~m}^{2}$
For two parallel links, $\quad A=576 \times 10^{-6} \mathrm{~m}^{2}$
(b) $\quad \sigma_{C E}=\frac{F_{C E}}{A}=\frac{-12.5 \times 10^{3}}{576 \times 10^{-6}}=-21.701 \times 10^{-6}$

$$
\sigma_{C E}=-21.7 \mathrm{MPa}
$$



## PROBLEM 1.8

Link AC has a uniform rectangular cross section $\frac{1}{8}$ in. thick and 1 in . wide. Determine the normal stress in the central portion of the link.

## SOLUTION

Use the plate together with two pulleys as a free body. Note that the cable tension causes at $1200 \mathrm{lb}-\mathrm{in}$. clockwise couple to act on the body.


$$
\begin{aligned}
+) \Sigma M_{B} & =0:-(12+4)\left(F_{A C} \cos 30^{\circ}\right)+(10)\left(F_{A C} \sin 30^{\circ}\right)-1200 \mathrm{lb}=0 \\
F_{A C} & =-\frac{1200 \mathrm{lb}}{16 \cos 30^{\circ}-10 \sin 30^{\circ}}=-135.500 \mathrm{lb}
\end{aligned}
$$

Area of link $A C$ :

$$
A=1 \mathrm{in} . \times \frac{1}{8} \text { in. }=0.125 \mathrm{in}^{2}
$$

Stress in link $A C: \quad \sigma_{A C}=\frac{F_{A C}}{A}=-\frac{135.50}{0.125}=1084 \mathrm{psi}=1.084 \mathrm{ksi}$


## PROBLEM 1.9

Knowing that the central portion of the link $B D$ has a uniform crosssectional area of $800 \mathrm{~mm}^{2}$, determine the magnitude of the load $\mathbf{P}$ for which the normal stress in that portion of $B D$ is 50 MPa ..

## SOLUTION

Draw free body diagram of link $A C$.


$$
\begin{aligned}
& F_{B D}=\sigma A \\
& =\left(50 \times 10^{6} \mathrm{~N} / \mathrm{m}^{2}\right)\left(800 \times 10^{-6} \mathrm{~m}^{2}\right) \\
& \\
& 40 \times 10^{3} \mathrm{~N} \\
& B D=\sqrt{(0.56 \mathrm{~m})^{2}+(1.92 \mathrm{~m})^{2}} \\
& \quad=2.00 \mathrm{~m}
\end{aligned}
$$

Free Body $A C$ :

$$
\begin{aligned}
+) M_{C} & =0: \quad \frac{0.56}{2.00}\left(40 \times 10^{3}\right)(1.4)+\frac{1.92}{2.00}\left(40 \times 10^{3}\right)(1.4)-P(0.7+1.4)=0 \\
P & =33.1 \times 10^{3} \mathrm{~N}
\end{aligned}
$$

$$
P=33.1 \mathrm{kN}
$$



## SOLUTION

Use bar $A B C$ as a free body.

(a) $\quad \theta=0$.
$+) \Sigma M_{A}=0: \quad\left(18 \sin 30^{\circ}\right)(4)-\left(12 \cos 30^{\circ}\right) F_{B D}=0$
$F_{B D}=3.4641 \mathrm{kips}$ (tension)
Area for tension loading: $\quad A=(b-d) t=\left(1-\frac{3}{8}\right)\left(\frac{1}{2}\right)=0.31250 \mathrm{in}^{2}$
Stress:

$$
\sigma=\frac{F_{B D}}{A}=\frac{3.4641 \mathrm{kips}}{0.31250 \mathrm{in}^{2}}
$$

(b) $\quad \theta=90^{\circ}$.
$+) \Sigma M_{A}=0: \quad-\left(18 \cos 30^{\circ}\right)(4)-\left(12 \cos 30^{\circ}\right) F_{B D}=0$
$F_{B D}=-6 \mathrm{kips}$ i.e. compression.
Area for compression loading: $\quad A=b t=(1)\left(\frac{1}{2}\right)=0.5 \mathrm{in}^{2}$
Stress:

$$
\sigma=\frac{F_{B D}}{A}=\frac{-6 \mathrm{kips}}{0.5 \mathrm{in}^{2}}
$$

$$
\sigma=12.00 \mathrm{ksi}
$$



## PROBLEM 1.11

The rigid bar $E F G$ is supported by the truss system shown. Knowing that the member $C G$ is a solid circular rod of 0.75 in. diameter, determine the normal stress in $C G$.

## SOLUTION



Using portion $E F G C B$ as free body,

$$
+\uparrow \Sigma F_{y}=0: \frac{3}{5} F_{A E}-3600=0, \quad F_{A E}=6000 \mathrm{lb}
$$

Use beam $E F G$ as free body.

$$
\begin{gathered}
+) \Sigma M_{F}=0:-(4) \frac{3}{5} F_{A E}+(4) \frac{3}{5} F_{C G}=0 \\
F_{A E}=F_{C G}=6000 \mathrm{lb}
\end{gathered}
$$

Normal stress in member $C G$
Area: $\quad A=\frac{\pi d^{2}}{4}=0.44179 \mathrm{in}^{2}$

$$
\sigma_{C G}=\frac{F}{A}=\frac{6000}{0.44179}=13580 \mathrm{psi}
$$

$$
\sigma_{C G}=13.58 \mathrm{ksi}
$$



## PROBLEM 1.12

The rigid bar $E F G$ is supported by the truss system shown. Determine the cross-sectional area of member $A E$ for which the normal stress in the member is 15 ksi .

## SOLUTION

Using portion $E F G C B$ as free body,


$$
+\uparrow \Sigma F_{y}=0: \quad \frac{3}{5} F_{A E}-3600=0, \quad F_{A E}=6000 \mathrm{lb}
$$

Normal stress in member $A E=15 \mathrm{ksi}$

$$
\begin{aligned}
\sigma_{A E} & =\frac{F}{A} \\
A & =\frac{F}{\sigma_{A E}}=\frac{6.00 \mathrm{kips}}{15 \mathrm{ksi}}
\end{aligned}
$$

$$
A=0.400 \mathrm{in}^{2}
$$



## PROBLEM 1.13

An aircraft tow bar is positioned by means of a single hydraulic cylinder connected by a $25-\mathrm{mm}$-diameter steel rod to two identical arm-and-wheel units $D E F$. The mass of the entire tow bar is 200 kg , and its center of gravity is located at $G$. For the position shown, determine the normal stress in the rod.


## PROBLEM 1.14

Two hydraulic cylinders are used to control the position of the robotic arm $A B C$. Knowing that the control rods attached at $A$ and $D$ each have a $20-\mathrm{mm}$ diameter and happen to be parallel in the position shown, determine the average normal stress in $(a)$ member $A E,(b)$ member $D G$.

## SOLUTION

Use member $A B C$ as free body.

$$
\begin{aligned}
& \text { Use member } A B C \\
& \text { as free body. }
\end{aligned}
$$


$+) \Sigma M_{B}=0: \quad(0.150) \frac{4}{5} F_{A E}-(0.600)(800)=0$

$$
F_{A E}=4 \times 10^{3} \mathrm{~N}
$$

Area of rod in member $A E$ is

$$
A=\frac{\pi}{4} d^{2}=\frac{\pi}{4}\left(20 \times 10^{-3}\right)^{2}=314.16 \times 10^{-6} \mathrm{~m}^{2}
$$

Stress in $\operatorname{rod} A E: \quad \quad \sigma_{A E}=\frac{F_{A E}}{A}=\frac{4 \times 10^{3}}{314.16 \times 10^{-6}}=12.7324 \times 10^{6} \mathrm{~Pa}$
(a) $\quad \sigma_{A E}=12.73 \mathrm{MPa}$

Use combined members $A B C$ and $B F D$ as free body.

$I_{F_{y}}$

$$
A=\frac{\pi}{4} d^{2}=\frac{\pi}{4}\left(20 \times 10^{-3}\right)^{2}=314.16 \times 10^{-6} \mathrm{~m}^{2}
$$

Stress in $\operatorname{rod} D G: \quad \sigma_{D G}=\frac{F_{D G}}{A}=\frac{-1500}{3.1416 \times 10^{-6}}=-4.7746 \times 10^{6} \mathrm{~Pa}$


## PROBLEM 1.15

Knowing that a force $\mathbf{P}$ of magnitude 50 kN is required to punch a hole of diameter $d=20 \mathrm{~mm}$ in an aluminum sheet of thickness $t=5 \mathrm{~mm}$, determine the average shearing stress in the aluminum at failure.

## SOLUTION

Area of failure in plate:

$$
\begin{aligned}
A & =\pi d t=\pi(0.020 \mathrm{~m})(0.005 \mathrm{~m}) \\
& =3.1416 \times 10^{-4} \mathrm{~m}^{2}
\end{aligned}
$$

Average shearing stress:

$$
\begin{aligned}
\tau_{\text {avg }} & =\frac{P}{A} \\
& =\frac{50 \times 10^{3} \mathrm{~N}}{3.1416 \times 10^{-4} \mathrm{~m}^{2}}
\end{aligned}
$$

$$
\tau_{\text {avg }}=159.2 \mathrm{MPa}
$$

## SOLUTION

Six areas must be sheared off when the joint fails. Each of these areas has dimensions $\frac{5}{8}$ in. $\times \frac{1}{2}$ in., its area being

$$
A=\frac{5}{8} \times \frac{1}{2}=\frac{5}{16} \mathrm{in}^{2}=0.3125 \mathrm{in}^{2}
$$

At failure, the force carried by each area is

$$
F=\tau A=(1.20 \mathrm{ksi})\left(0.3125 \mathrm{in}^{2}\right)=0.375 \mathrm{kips}
$$

Since there are six failure areas,

$$
P=6 F=(6)(0.375) \quad P=2.25 \mathrm{kips}
$$



## PROBLEM 1.17

When the force $\mathbf{P}$ reached 1600 lb , the wooden specimen shown failed in shear along the surface indicated by the dashed line. Determine the average shearing stress along that surface at the time of failure.

## SOLUTION

Area being sheared:
Force:
Shearing stress:

$$
A=3 \mathrm{in} . \times 0.6 \mathrm{in} .=1.8 \mathrm{in}^{2}
$$

$$
P=1600 \mathrm{lb}
$$

$$
\tau=\frac{P}{A}-\frac{1600 \mathrm{lb}}{1.8 \mathrm{in}^{2}}=8.8889 \times 10^{2} \mathrm{psi}
$$

$$
\tau=889 \mathrm{psi}
$$



## PROBLEM 1.18

A load $\mathbf{P}$ is applied to a steel rod supported as shown by an aluminum plate into which a $12-\mathrm{mm}$-diameter hole has been drilled. Knowing that the shearing stress must not exceed 180 MPa in the steel rod and 70 MPa in the aluminum plate, determine the largest load $\mathbf{P}$ that can be applied to the rod.

## SOLUTION

For steel:

$$
\begin{aligned}
A_{1} & =\pi d t=\pi(0.012 \mathrm{~m})(0.010 \mathrm{~m}) \\
& =376.99 \times 10^{-6} \mathrm{~m}^{2} \\
\tau_{1}=\frac{P}{A} \therefore P & =A_{1} \tau_{1}=\left(376.99 \times 10^{-6} \mathrm{~m}^{2}\right)\left(180 \times 10^{6} \mathrm{~Pa}\right) \\
& =67.858 \times 10^{3} \mathrm{~N}
\end{aligned}
$$

For aluminum:

$$
A_{2}=\pi d t=\pi(0.040 \mathrm{~m})(0.008 \mathrm{~m})=1.00531 \times 10^{-3} \mathrm{~m}^{2}
$$

$$
\tau_{2}=\frac{P}{A_{2}} \therefore P=A_{2} \tau_{2}=\left(1.00531 \times 10^{-3} \mathrm{~m}^{2}\right)\left(70 \times 10^{6} \mathrm{~Pa}\right)=70.372 \times 10^{3} \mathrm{~N}
$$

Limiting value of $P$ is the smaller value, so


## PROBLEM 1.19

The axial force in the column supporting the timber beam shown is $P$ $=20$ kips. Determine the smallest allowable length $L$ of the bearing plate if the bearing stress in the timber is not to exceed 400 psi .

## SOLUTION

Bearing area: $A_{b}=L w$

$$
\begin{array}{rlr}
\sigma_{b} & =\frac{P}{A_{b}}=\frac{P}{L w} \\
L & =\frac{P}{\sigma_{b} w}=\frac{20 \times 10^{3} \mathrm{lb}}{(400 \mathrm{psi})(6 \mathrm{in} .)}=8.33 \mathrm{in} . & L=8.33 \mathrm{in} .
\end{array}
$$



## PROBLEM 1.20

Three wooden planks are fastened together by a series of bolts to form a column. The diameter of each bolt is 12 mm and the inner diameter of each washer is 16 mm , which is slightly larger than the diameter of the holes in the planks. Determine the smallest allowable outer diameter $d$ of the washers, knowing that the average normal stress in the bolts is 36 MPa and that the bearing stress between the washers and the planks must not exceed 8.5 MPa .

## SOLUTION

Bolt:

$$
A_{\text {Bolt }}=\frac{\pi d^{2}}{4}=\frac{\pi(0.012 \mathrm{~m})^{2}}{4}=1.13097 \times 10^{-4} \mathrm{~m}^{2}
$$

Tensile force in bolt: $\quad \sigma=\frac{P}{A} \Rightarrow P=\sigma A$

$$
\begin{aligned}
& =\left(36 \times 10^{6} \mathrm{~Pa}\right)\left(1.13097 \times 10^{-4} \mathrm{~m}^{2}\right) \\
& =4.0715 \times 10^{3} \mathrm{~N}
\end{aligned}
$$

Bearing area for washer:

$$
\begin{aligned}
& A_{w}=\frac{\pi}{4}\left(d_{o}^{2}-d_{i}^{2}\right) \\
& A_{w}=\frac{P}{\sigma_{B R G}}
\end{aligned}
$$

Therefore, equating the two expressions for $A_{w}$ gives

$$
\begin{aligned}
\frac{\pi}{4}\left(d_{o}^{2}-d_{i}^{2}\right) & =\frac{P}{\sigma_{B R G}} \\
d_{o}^{2} & =\frac{4 P}{\pi \sigma_{B R G}}+d_{i}^{2} \\
d_{o}^{2} & =\frac{4}{\pi} \frac{\left(4.0715 \times 10^{3} \mathrm{~N}\right)}{\left(8.5 \times 10^{6} \mathrm{~Pa}\right)}+(0.016 \mathrm{~m})^{2} \\
d_{o}^{2} & =8.6588 \times 10^{-4} \mathrm{~m}^{2} \\
d_{o} & =29.426 \times 10^{-3} \mathrm{~m}
\end{aligned}
$$

$$
d_{o}=29.4 \mathrm{~mm}
$$



## SOLUTION

(a) Bearing stress on concrete footing.

$$
\begin{aligned}
& P=40 \mathrm{kN}=40 \times 10^{3} \mathrm{~N} \\
& A=(100)(120)=12 \times 10^{3} \mathrm{~mm}^{2}=12 \times 10^{-3} \mathrm{~m}^{2} \\
& \sigma=\frac{P}{A}=\frac{40 \times 10^{3}}{12 \times 10^{-3}}=3.3333 \times 10^{6} \mathrm{~Pa}
\end{aligned}
$$

$$
3.33 \mathrm{MPa}
$$

(b) Footing area. $P=40 \times 10^{3} \mathrm{~N} \quad \sigma=145 \mathrm{kPa}=45 \times 10^{3} \mathrm{~Pa}$

$$
\sigma=\frac{P}{A} \quad A=\frac{P}{\sigma}=\frac{40 \times 10^{3}}{145 \times 10^{3}}=0.27586 \mathrm{~m}^{2}
$$

Since the area is square, $A=b^{2}$

$$
b=\sqrt{A}=\sqrt{0.27586}=0.525 \mathrm{~m}
$$

$$
b=525 \mathrm{~mm}
$$



## PROBLEM 1.22

The axial load $P=240$ kips, supported by a $\mathrm{W} 10 \times 45$ column, is distributed to a concrete foundation by a square base plate as shown. Determine the size of the base plate for which the average bearing stress on the concrete is 750 psi .

## SOLUTION

$$
\begin{aligned}
& \sigma=\frac{P}{A} \text { or } \\
& A=\frac{P}{\sigma} \\
&=\frac{240 \times 10^{3} \mathrm{lb}}{750 \mathrm{psi}} \\
&= 320 \mathrm{in}^{2}
\end{aligned}
$$

Since the plate is square,

$$
\begin{aligned}
& A=b^{2} \\
& b=\sqrt{320 \mathrm{in}^{2}}
\end{aligned}
$$



## PROBLEM 1.23

An axial load $\mathbf{P}$ is supported by a short $\mathrm{W} 8 \times 40$ column of crosssectional area $A=11.7 \mathrm{in}^{2}$ and is distributed to a concrete foundation by a square plate as shown. Knowing that the average normal stress in the column must not exceed 30 ksi and that the bearing stress on the concrete foundation must not exceed 3.0 ksi , determine the side $a$ of the plate that will provide the most economical and safe design.

## SOLUTION

For the column, $\sigma=\frac{P}{A}$ or

$$
P=\sigma A=(30)(11.7)=351 \mathrm{kips}
$$

For the $a \times a$ plate, $\sigma=3.0 \mathrm{ksi}$

$$
A=\frac{P}{\sigma}=\frac{351}{3.0}=117 \mathrm{in}^{2}
$$

Since the plate is square, $A=a^{2}$

$$
a=\sqrt{A}=\sqrt{117}
$$

$$
a=10.82 \mathrm{in} .
$$



## PROBLEM 1.24

A 6-mm-diameter pin is used at connection $C$ of the pedal shown. Knowing that $P=500 \mathrm{~N}$, determine $(a)$ the average shearing stress in the pin, $(b)$ the nominal bearing stress in the pedal at $C$, $(c)$ the nominal bearing stress in each support bracket at $C$.

## SOLUTION

Since $B C D$ is a 3-force member, the reaction at $C$ is directed toward $E$, the intersection of the lines of act of the other two forces.


From geometry,

$$
C E=\sqrt{300^{2}+125^{2}}=325 \mathrm{~mm}
$$

From the free body diagram of $B C D$,
(a)

$$
\begin{aligned}
+\uparrow \Sigma F_{y} & =0: \frac{125}{325} C-P=0 \quad C=2.6 P=2.6(500 \mathrm{~N})=1300 \mathrm{~N} \\
\tau_{\text {pin }} & =\frac{\frac{1}{2} C}{A_{P}}=\frac{\frac{1}{2} C}{\frac{\pi}{4} d^{2}}=\frac{2 C}{\pi d^{2}} \\
\tau_{\text {pin }} & =\frac{2(1300 \mathrm{~N})}{\pi\left(6 \times 10^{-3} \mathrm{~m}\right)^{2}}=23.0 \times 10^{6} \mathrm{~Pa}
\end{aligned}
$$

$$
\tau_{\text {pin }}=23.0 \mathrm{MPa}
$$

(b) $\quad \sigma_{b}=\frac{C}{A_{b}}=\frac{C}{d t}=\frac{(1300)}{\left(6 \times 10^{-3}\right)\left(9 \times 10^{-3}\right)}=24.1 \times 10^{6} \mathrm{~Pa}$

$$
\sigma_{b}=24.1 \mathrm{MPa}
$$

(c) $\sigma_{b}=\frac{\frac{1}{2} C}{A_{b}}=\frac{C}{2 d t}=\frac{(1300)}{2\left(6 \times 10^{-3}\right)\left(9 \times 10^{-3}\right)}=21.7 \times 10^{6} \mathrm{~Pa}$


## PROBLEM 1.25

Knowing that a force $\mathbf{P}$ of magnitude 750 N is applied to the pedal shown, determine ( $a$ ) the diameter of the pin at $C$ for which the average shearing stress in the pin is $40 \mathrm{MPa},(b)$ the corresponding bearing stress in the pedal at $C,(c)$ the corresponding bearing stress in each support bracket at $C$.

## SOLUTION

Since $B C D$ is a 3-force member, the reaction at $C$ is directed toward $E$, the intersection of the lines of action of the other two forces.


From geometry, $C E=\sqrt{300^{2}+125^{2}}=325 \mathrm{~mm}$

From the free body diagram of $B C D$,
(a)

$$
\begin{aligned}
& +\uparrow \Sigma F_{y}=0: \frac{125}{325} C-P=0 \quad C=2.6 P=2.6(750 \mathrm{~N})=1950 \mathrm{~N} \\
& \tau_{\text {pin }}=\frac{\frac{1}{2} C}{A_{P}}=\frac{\frac{1}{2} C}{\frac{\pi}{4} d^{2}}=\frac{2 C}{\pi d^{2}} \\
& d=\sqrt{\frac{2 C}{\pi \tau_{\text {pin }}}}=\sqrt{\frac{2(1950 \mathrm{~N})}{\pi\left(40 \times 10^{6} \mathrm{~Pa}\right)}}=5.57 \times 10^{-3} \mathrm{~m}
\end{aligned}
$$

(b) $\quad \sigma_{b}=\frac{C}{A_{b}}=\frac{C}{d t}=\frac{(1950)}{\left(5.57 \times 10^{-3}\right)\left(9 \times 10^{-3}\right)}=38.9 \times 10^{6} \mathrm{~Pa}$

$$
\sigma_{b}=38.9 \mathrm{MPa}
$$

(c) $\sigma_{b}=\frac{\frac{1}{2} C}{A_{b}}=\frac{C}{2 d t}=\frac{(1950)}{2\left(5.57 \times 10^{-3}\right)\left(5 \times 10^{-3}\right)}=35.0 \times 10^{6} \mathrm{~Pa}$


## PROBLEM 1.26

The hydraulic cylinder $C F$, which partially controls the position of rod $D E$, has been locked in the position shown. Member $B D$ is 15 mm thick and is connected at C to the vertical rod by a 9 -mm-diameter bolt. Knowing that $P=2 \mathrm{kN}$ and $\theta=75^{\circ}$, determine (a) the average shearing stress in the bolt, $(b)$ the bearing stress at $C$ in member $B D$.

## SOLUTION

Free Body: Member BD.


$$
\begin{aligned}
&+) \Sigma M_{c}= 0: \quad \frac{40}{41} F_{A B}\left(100 \cos 20^{\circ}\right)-\frac{9}{4} F_{A B}\left(100 \sin 20^{\circ}\right) \\
&-(2 \mathrm{kN}) \cos 75^{\circ}\left(175 \sin 20^{\circ}\right)-(2 \mathrm{kN}) \sin 75^{\circ}\left(175 \cos 20^{\circ}\right)=0 \\
& \frac{100}{41} F_{A B}\left(40 \cos 20^{\circ}-9 \sin 20^{\circ}\right)=(2 \mathrm{kN})(175) \sin \left(75^{\circ}+20^{\circ}\right) \\
& F_{A B}=4.1424 \mathrm{kN} \\
&+\Sigma F_{x}= 0: \quad C_{x}-\frac{9}{41}(4.1424 \mathrm{kN})+(2 \mathrm{kN}) \cos 75^{\circ}=0 \\
& C_{x}= 0.39167 \mathrm{kN} \\
&+{ }^{4} \Sigma F_{y}= 0: \quad C_{y}-\frac{40}{41}(4.1424 \mathrm{kN})-(2 \mathrm{kN}) \sin 75^{\circ}=0 \\
& C_{y}=5.9732 \mathrm{kN} \\
& \mathrm{C}=5.9860 \mathrm{kN} \mathrm{Z} 86.2^{\circ}
\end{aligned}
$$

(a) $\tau_{\text {ave }}=\frac{C}{A}=\frac{5.9860 \times 10^{3} \mathrm{~N}}{\pi(0.0045 \mathrm{~m})^{2}}=94.1 \times 10^{6} \mathrm{~Pa}=94.1 \mathrm{MPa}$
(b) $\quad \tau_{b}=\frac{C}{t d}=\frac{5.9860 \times 10^{3} \mathrm{~N}}{(0.015 \mathrm{~m})(0.009 \mathrm{~m})}=44.3 \times 10^{6} \mathrm{~Pa}=44.3 \mathrm{MPa}$


## PROBLEM 1.27

For the assembly and loading of Prob. 1.7, determine (a) the average shearing stress in the pin at $B,(b)$ the average bearing stress at $B$ in member $B D$, (c) the average bearing stress at $B$ in member $A B C$, knowing that this member has a $10 \times 50-\mathrm{mm}$ uniform rectangular cross section.

PROBLEM 1.7 Each of the four vertical links has an $8 \times 36-\mathrm{mm}$ uniform rectangular cross section and each of the four pins has a $16-\mathrm{mm}$ diameter. Determine the maximum value of the average normal stress in the links connecting (a) points $B$ and $D,(b)$ points $C$ and $E$.

## SOLUTION

Use bar $A B C$ as a free body.

$+) \Sigma M_{C}=0:(0.040) F_{B D}-(0.025+0.040)\left(20 \times 10^{3}\right)=0$
$F_{B D}=32.5 \times 10^{3} \mathrm{~N}$
(a) Shear pin at $B . \quad \tau=\frac{F_{B D}}{2 A}$ for double shear
where

$$
\begin{aligned}
& A=\frac{\pi}{4} d^{2}=\frac{\pi}{4}(0.016)^{2}=201.06 \times 10^{-6} \mathrm{~m}^{2} \\
& \tau=\frac{32.5 \times 10^{3}}{(2)\left(201.06 \times 10^{-6}\right)}=80.822 \times 10^{6} \mathrm{~Pa}
\end{aligned}
$$

$$
\tau=80.8 \mathrm{MPa}
$$

(b) Bearing: link $B D$.

$$
A=d t=(0.016)(0.008)=128 \times 10^{-6} \mathrm{~m}^{2}
$$

$$
\sigma_{b}=\frac{\frac{1}{2} F_{B D}}{A}=\frac{(0.5)\left(32.5 \times 10^{3}\right)}{128 \times 10^{-6}}=126.95 \times 10^{6} \mathrm{~Pa} \quad \sigma_{b}=127.0 \mathrm{MPa}
$$

(c) Bearing in $A B C$ at $B . \quad A=d t=(0.016)(0.010)=160 \times 10^{-6} \mathrm{~m}^{2}$

$$
\sigma_{b}=\frac{F_{B D}}{A}=\frac{32.5 \times 10^{3}}{160 \times 10^{-6}}=203.12 \times 10^{6} \mathrm{~Pa} \quad \sigma_{b}=203 \mathrm{MPa}
$$



## SOLUTION

Use one fork as a free body.


$$
\begin{aligned}
&+\Sigma M_{B}=0: \quad 24 E-(20)(1500)=0 \\
& E=1250 \mathrm{lb} \longrightarrow \\
&+\Sigma F_{x}=0: \quad E+B_{x}=0 \\
& B_{x}=-E \\
& B_{x}=1250 \mathrm{lb} \longleftarrow \\
&+\uparrow \Sigma F_{y}=0: \quad B_{y}-1500=0 \quad B_{y}=1500 \mathrm{lb} \\
& B=\sqrt{B_{x}^{2}+B_{y}^{2}}=\sqrt{1250^{2}+1500^{2}}=1952.56 \mathrm{lb}
\end{aligned}
$$

(a) Shearing stress in pin at $B$.

$$
\begin{aligned}
A_{\mathrm{pin}} & =\frac{\pi}{4} d_{\mathrm{pin}}^{2}=\frac{\pi}{4}\left(\frac{1}{2}\right)^{2}=0.196350 \mathrm{in}^{2} \\
\tau & =\frac{B}{A_{\mathrm{pin}}}=\frac{1952.56}{0.196350}=9.94 \times 10^{3} \mathrm{psi}
\end{aligned}
$$

$$
\tau=9.94 \mathrm{ksi}
$$

(b) Bearing stress at $B$.

$$
\sigma=\frac{B}{d t}=\frac{1952.56}{\left(\frac{1}{2}\right)\left(\frac{5}{8}\right)}=6.25 \times 10^{3} \mathrm{psi}
$$

$$
\sigma=6.25 \mathrm{ksi}
$$

